

**PRECAST SEGMENTAL POST-TENSIONED
CONCRETE BRIDGE COLUMNS FOR SEISMIC
REGIONS**

by

Yu-Chen Ou

June 2007

**A dissertation submitted to the Faculty of the Graduate School of State
University of New York at Buffalo in partial fulfillment of the
requirements for the degree of**

Doctor of Philosophy

Department of Civil, Structural, and Environmental Engineering

ABSTRACT

Precast concrete bridge construction has been proved to be an efficient solution in accelerating bridge construction and minimizing traffic disruption. However, due to concerns with the seismic performance of such type of construction, its application in seismic regions is limited. This research presents results of the development of precast segmental post-tensioned concrete bridge columns for use in seismic regions. The developed bridge columns adopted unbonded post-tensioning systems to decrease prestress loss due to strong seismic events. In addition, to increase hysteretic energy dissipation, mild steel energy dissipation bars (ED bars) which are continuous across the segment joints are added to the columns. Moreover, the ED bars are additionally unbonded at the critical joint to avoid premature fracture.

A simplified analytical model for static pushover analysis and a three-dimensional detailed finite element model for cyclic analysis of the proposed bridge columns are developed in this research. In addition, a stiffness degrading hysteretic model is proposed for response-history analysis. With the analytical models, a parametric study is conducted to examine the seismic performance of the proposed columns with different design parameters.

A two-phase experimental program is designed to verify the findings of the analytical study and to address constructability issues of the proposed segmental columns. The first phase focuses on testing of the ED bars at the critical joint. A methodology is proposed to design the additional unbonded length for the bars, taking into account low cycle fatigue of steel and progressive damage of bond. The second phase is testing of seven large scale segmental column specimens. Each specimen has a foundation, four precast column segments with hollow cross sections and a precast cap beam with a total height of 5.7 m (18.7 ft). Four specimens are tested with cyclic loading and three with pseudo-dynamic loading. The test results show that three types of ductile precast segmental columns are successfully developed with different hysteretic characteristics in terms of energy dissipation, lateral strength and residual displacement.

ACKNOWLEDGEMENTS

First of all, I would like to thank to my advisor, Professor George C. Lee for his guidance to this research and financial support over the past four years. I learned a great deal from him not only from his knowledge and experiences in civil engineering but also from the wisdom that he gained from his more than four decades of academic career. I also would like to thank to Professor Amjad Aref, Professor Stuart S. Chen and Professor Andre Filiatrault, my Ph.D. committee members, for their advice to this research. The advice from Dr. Methee Chiewanichakorn on developing the analytical models for this research is greatly acknowledged. In addition, I would like to thank to Dr. Il-Sang Ahn for his advice to this research.

I would like to thank to Kuo-Chun Chang, Professor and Chairman of the Department of Civil Engineering of the National Taiwan University, for his advice throughout this research. His support is the key to the success of the experimental study conducted in this research. I also would like to thank to Dr. Jui-Chen Wang for providing valuable information and guidance on developing the new concepts for precast segmental bridge columns for seismic regions. Mr. Ping-Hsiung Wang worked relentlessly throughout the experimental study. Without his effort, the experiment could not have been completed in only one year. Mr. Mu-Sen Tsai also played a significantly role in assisting the experimental study. Thank for their effort.

I must thank to the Federal Highway Administration for financially supporting this research under grant No. DTSH61-98-C-00094. In addition, I would like to thank to the Taiwan's National Center for Research on Earthquake Engineering (NCREE) for funding the experimental study conducted in this research.

At last, I would like to express my deep thank to my parents for their continuous support for all of these years. In addition, I would like to thank to my girlfriend, Ms. Huei-An Chuang for her patience and love over the past years from my Master research to Ph.D. research.

TABLE OF CONTENTS

ABSTRACT	i
ACKNOWLEDGEMENTS	ii
TABLE OF CONTENTS	iii
LIST OF TABLES	vii
LIST OF FIGURES	viii
CHAPTER 1 INTRODUCTION	1
1.1 Motivation.....	1
1.2 Background.....	7
1.3 Objectives	10
CHAPTER 2 SEISMIC DESIGN CONCEPTS FOR SEGMENTAL COLUMNS .	11
2.1 Continuity in Potential Plastic Hinge Regions.....	11
2.2 Concrete Confinement for Potential Plastic Hinge Regions.....	13
2.3 Hysteretic Behavior	13
2.3.1. Jointed Behavior with High Energy Dissipation and Residual Displacement	13
2.3.2. Jointed Behavior with Minimal Energy Dissipation and Residual Displacement	14
2.3.3. Jointed Behavior with Moderate Energy Dissipation and Small Residual Displacement	16
2.4 Bonded or Unbonded Post-tensioning	18
2.5 Shape of Column Section.....	20
2.6 Other Concepts.....	21
2.6.1. Shear Keys at Column Segment Joints	21
2.6.2. Post-Tensioning Strand or Bar System	21
2.6.3. Time-Dependant Effects	22
2.6.4. Match Cast and Epoxied Joint	22
CHAPTER 3 ANALYTICAL STUDY.....	24
3.1 Simplified Analytical Model.....	24
3.1.1. Background	24
3.1.2. Assumptions.....	25
3.1.3. Moment-curvature-axial Force Analysis	28

3.2 Finite Element Model	32
3.2.1. Modeling Approach	32
3.2.1.1. Footing and Column Segments.....	32
3.2.1.2. Post-Tensioning Tendons.....	33
3.2.1.3. Energy Dissipation Bars	33
3.2.1.4. Segment Joint Interfaces.....	34
3.2.1.5. Analysis Procedure	34
3.2.2. Verification of Finite Element Model.....	34
3.3 Parametric Study.....	37
3.3.1. Prototype Column Design.....	37
3.3.1.1. Slenderness Ratio of Column Panel Wall.....	37
3.3.1.2. Confinement Reinforcement for Potential Plastic Hinge Regions	38
3.3.1.3. Shear Capacity across Column Segment Joints	39
3.3.1.4. Shear Capacity of Column Segments	40
3.3.1.5. Upper and Lower Limits on Amount of Longitudinal Reinforcement	41
3.3.1.6. Energy Dissipation Bars (ED Bars)	43
3.3.2. Nomenclature.....	45
3.3.3. Verification of Simplified Analytical Model.....	46
3.3.4. Analyses using Simplified Analytical Model	52
3.3.5. Analyses using 3D FEM	56
3.3.6. Response-history Analyses	60
CHAPTER 4 CRITICAL JOINT TESTING	67
4.1 Background and Objectives	67
4.2 Specimen Description	70
4.2.1. Specimen Design	70
4.2.2. Design Parameters of Specimens.....	72
4.2.2.1. Monotonic Loading Tests – Part 1	72
4.2.2.2. Monotonic Loading Tests – Part 2	73
4.2.2.2.1. Foundation Specimens	73
4.2.2.2.2. Base Segment Specimens	76
4.2.2.3. Cyclic Loading Tests	76
4.2.2.3. Grouting of Foundation Specimens	77
4.2.3. Test Setup and Instrumentation	79
4.3.1. Strain Gauges	79
4.3.2. Monotonic Loading.....	80
4.3.3. Cyclic Loading.....	81
4.4 Test Results	84
4.4.1. Monotonic Loading Tests – Phase 1	84
4.4.2. Monotonic Loading Tests – Phase 2	88
4.4.2.1. Bonded Length of 4 d_b	88
4.4.2.2. Bonded Length of 16 d_b	90
4.4.2.3. Bonded Length of 24 d_b	92

4.4.2.4. Bonded Length of 24 d_b plus an Additional Unbonded Length of 8 d_b	93
4.4.2.5. Bonded Length of 900 mm	96
4.4.3. Cyclic Loading Tests	98
4.4.3.1. Bonded Length of 4 d_b	98
4.4.3.2. Bonded Length of 16 d_b	100
4.4.3.3. Bonded Length of 24 d_b	100
4.4.3.4. Bonded Length of 24 d_b plus an Additional Unbonded Length of 8 d_b	101
4.4.3.5. Bonded Length of 900 mm	102
4.5 Design of Additional Unbonded Length.....	103
4.5.1. Equivalent Unbonded Length	103
4.5.2. Design of Additional Unbonded Length for Column Specimens	105
CHAPTER 5 SEGMENT COLUMN TESTING.....	111
5.1 Background and Objectives	111
5.2 Specimen Description	112
5.2.1. Design Concepts	112
5.2.2. Specimen Design	115
5.2.3. Assembly Method	121
5.2.4. Instrumentation	125
5.2.5. Test Setup and Loading Scheme.....	128
5.3 Results of Cyclic Loading Tests	132
5.3.1. Test Process	132
5.3.2. Damage Distribution.....	137
5.3.3. Hysteretic Behavior	140
5.3.4. Hysteretic Energy Dissipation	144
5.3.5. Residual Drift.....	147
5.3.6. Joint Opening.....	148
5.3.7. Rotation Increment.....	149
5.3.8. Prestressing Force	151
5.3.9. Strains in ED Bars.....	152
5.3.10. Strains in Mild Steel Reinforcement inside Column Segments.....	157
5.4. Results of Pseudo-Dynamic Loading Tests	162
5.4.1. General Observation	162
5.4.2. Damage Distribution.....	169
5.4.3. Hysteretic Behavior	172
5.4.4. Energy	178
5.4.5. Residual Drift under Cyclic Loading.....	180
5.4.6. Joint Opening.....	181
5.4.7. Rotation Increment.....	187
5.4.8. Prestressing Force	189
5.4.9. Strains in ED bars	192
5.4.10. Strains in Mild Steel Reinforcement inside Column Segments.....	196
5.5. Comparisons of Experimental Results to Analytical Studies	201

CHAPTER 6 SUMMARY AND CONCLUSIONS.....	204
6.1 Summary	204
6.2 Conclusions.....	206
6.2.1. Analytical Study.....	206
6.2.2. Critical Joint Testing.....	207
6.2.3. Segment Column Testing.....	208
6.2.3.1. Cyclic Loading Tests	208
6.2.3.2. Pseudo-Dynamic Loading Tests	209
6.3 Future Research Work	210
REFERENCES.....	212